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Name: Don Bell Signs
Project: City of South Daytona (2200807) - Gateway Entry Sign

Description: Additional Details

- Specifications:
- | | |
|---|--|
| <p>1. 2020 Florida Building Code, 7th Edition</p> <p>2. ACI 318-08, ASCE 7-16</p> <p>3. ASTM F1554 Grade 36, ASTM A307 Anchor Bolts, (Heavy Hex on Bottom, not "L" bolts, UNO)</p> <p>4. ASTM A36 Structural Steel</p> <p>5. ASTM A325 Connection Bolts, Snug Tight</p> <p>6. ASTM A500 Grade B, Structural Steel Tubing, $F_y = 46$ ksi</p> <p>7. ASTM A449 Hex Cap Screws, Bolts & Studs, Steel, Heat Treated, $F_y = 120, 105, 90$ minimum</p> <p>8. Will comply with National Electrical Coded (NEC) 2017</p> | <p>8. ASTM 6053, 6061-T6 Structural Aluminum Tubing, $F_y = 20$ ksi min.</p> <p>9. ASTM A53, Grade B, Type E or S, Structural Piping, $F_y = 35$ ksi</p> <p>10. Rebar, Grade 60 for #6 or Larger, Grade 40 for #5 or Smaller</p> <p>11. ASTM A992 / A572 Grade 50 - Standard I-Beams, $F_y = 50$ ksi</p> <p>12. ASTM A307 Carbon Steel Bolts & Studs</p> <p>13. ASTM C-920 Elastomeric Joint Sealant</p> <p>14. Digital Signatures (F.A.C. 61G15-23.004): This item has been electronically signed and sealed by Dustin DiPersia, PE, on this date using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.</p> |
|---|--|

CERTIFICATION: To the best of my knowledge, I certify this analysis meets structural requirements of: 2020 Florida Building Code, 7th Edition

LIMITATION: Valid for two (2) signs, at specified location. In case of conflict, structural requirements, scope of work, and installer, mfg, owner responsibilities control.

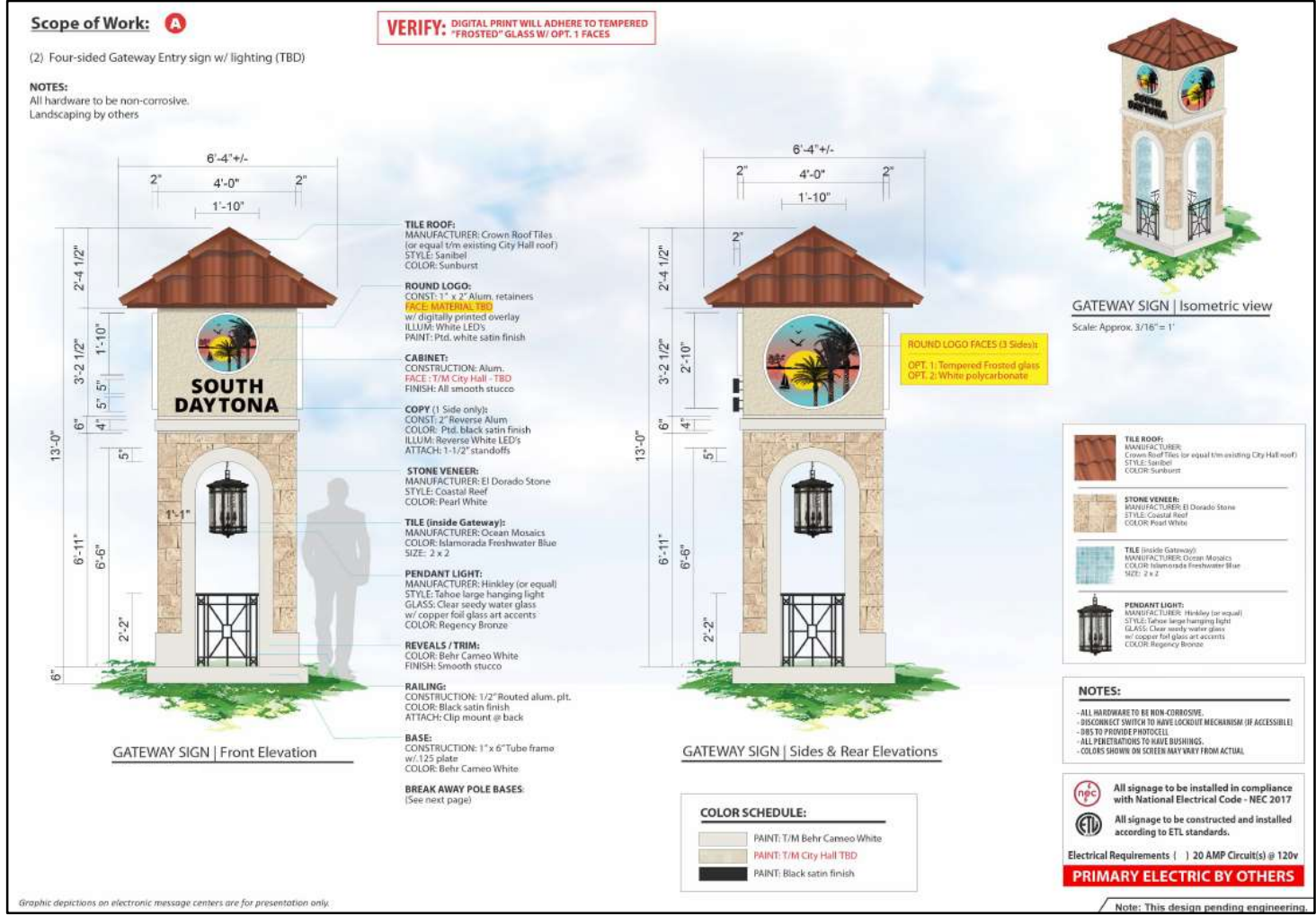
THIS SEAL FOR STRUCTURAL ONLY
DUSTIN DIPERSIA, P.E. FL 77276
CERTIFICATE OF AUTHORIZATION: 33209

Printed Date: 3/9/2023
Drawn By: Dustin DiPersia, P.E.
Checked By: Dustin DiPersia, P.E.

Special Notes:
1. Sign Location: US 1, South Daytona, FL
2. Column Specs: 4.50" O.D. X 0.25", $f_y = 29.1$ ksi - 6061-T6 Aluminum
3. Requ. Foot. Size = 2.00 ft dia. @ #####
4. Assumed soil bearing capacity = 2200.00 psf, sides = 200 psf per foot depth (Field Verify)

Signature:
Date: 3/9/2023

Details:





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FDOT Breakaway Base

STUB/SLEEVE & BASE PLATE DETAILS

Welded Or Sandcast

STUB DETAIL

WELDED STUB BASE

DETAIL 'A'

BOLTED STUB/SLEEVE BASE

SLIP BASE AND FOUNDATION DETAIL
(Non-Fragile Column, Typ.)

SLIP BASE AND FOUNDATION DETAIL IN CONCRETE
(Non-Fragile Column In Crossovers, Medians & Sidewalks)

NOTES:

- Foundation Notes for Slip Base.
 - See FOUNDATION TABLE on Sheet 3 for foundation entrenchment depth and slab length.
- Slip Base Fabrication Notes.
 - The difference between the O.D. of the post and I.D. of the Sleeve must be $\frac{1}{16}$ " or less.
 - The WELDED STUB BASE and lower STUB/SLEEVE BASE PLATE may be fabricated using galvanized steel as an option to aluminum. The upper portion of the SLIP BASE must be aluminum.
 - Either a Welded Stub Base or Bolted Stub/Sleeve Base may be used in Slip Base.
 - For cast base plates bolted to foundation stubs, use a foundation slab the same size as the sign column (slab).
- Slip-Base Assembly Instructions.
 - Assemble the Slip Base as follows:
 - Insert Post into Sleeve and connect using 2 - $\frac{1}{2}$ " diameter Sleeve Bolts.
 - Assemble top base plate to bottom Base Plate using Base Bolts (high strength) with 3 washers per bolt. (See Detail 'A').
 - Place one washer on each Base Bolt between the bottom Base Plate and the Bolt Keeper Plate.
 - Place the next washer between the Bottom Base Plate and the Bolt Keeper Plate.
 - Use brass or galvanized steel shims to plumb the post.
 - Add the top base plate section.
 - Place the third washer between the Top Base Plate and the Nut.
 - Orient the Bolt Keeper Plates in the Direction of Traffic.
 - Tighten Base Bolts as follows:
 - Tighten Base Bolts to the maximum possible with a 12" to 15" wrench (this will bend the washers and shims and clear the bolt threads).
 - Loosen each Base Bolt one turn.
 - Under the supervision of the Engineer, use a calibrated wrench to tighten bolts to the torque prescribed in the SLIP BASE DETAILS Table. Over tightened Base Bolts are not permitted.
 - Distort bolt threads at the junction with nuts to prevent loosening. Repair damaged galvanizing.
 - Obtain a tight sleeve connection by placing 4 galvanized steel shims between the column (post) and sleeve. Space the shims evenly around the perimeter of the column (1 between each bolt hole & total 4 total). Use shims that are 1" shorter than the height of the sleeve.

SLIP BASE AND FOUNDATION DETAILS

NOT TO SCALE

Drawing Sheet S-3



Name:	Don Bell Signs		
Project:	City of South Daytona (2200807) - Gateway Entry Sign		
Description:	Calculate Wind Loads		
Sign Dimensions:	1. Sign Height, H, Z & s =	13.00 ft	if h = 0, s = h
	2. Width, B =	4.00 ft	
	3. Height off Ground, h =	0.00 ft	

Analysis:	1. Wind Loads, ASCE 7-16		
	>Wind Force		
	a) <u>Velocity Pressure, VP</u> =	$0.00256 \times K_z \times K_{zt} \times K_d \times V^2 \times I$, where	
		K_z = velocity pressure exposure coefficient =	0.85
		K_{zt} = topographic speed up factor =	1.00
		K_d = wind directional factor =	0.85
		V = wind velocity	140 mph
		I = Importance Factor =	1.00
	For Z < 15 ft:	$K_z = 2.01 \left(\frac{15}{Z_g} \right)^{2/\alpha}$	
		, where	Z = height above ground level
	For $15 \leq Z \leq Z_g$:	$K_z = 2.01 \left(\frac{Z}{Z_g} \right)^{2/\alpha}$	Z_g & α = terrain exposure constants (ASCE 7-16)
	Therefore....	Velocity Pressure =	36.20 psf
	b) <u>Factored Wind Pressure, WP</u> =	$VP \times G \times C_f$, where	
		G = gust-effect factor (0.85 for rigid struct) =	0.85
		(otherwise use Equ. 26.9-6, ASCE 7-16)	
		C_f = force coeff (Fig. 29.4-1, ASCE 7-16) =	1.70
		a) aspect ratio = B/s =	0.31
		b) clearance ration = s/h =	1.00
	Therefore....	Factored Wind Pressure =	52.32 psf
	c) <u>Wind Force, WF</u> =	$WP \times A \times \frac{x^2}{L^2}$, where	
	Therefore....	Wind Force (Shear Force) =	1209.07 lbs
		Sign Weight (Axial Force) =	360.15 lbs
	>Moment At Grade		
	a) <u>Moment, M</u> =	$WF \times h$, where	
		h = moment arm = 0.5H =	6.50 ft
	Therefore....	Moment at Sign Bottom =	10.20 kips-ft
	2. Sign Column Bending, S (Section Modulus)		
	S =	$\frac{M}{f_b \times NC}$	
		M = Moment =	10.20 kips-ft
		f_b = yield strength =	29.1 ksi
		NC = # of Columns =	4.00
	Therefore....	Bending, S_{req} =	1.051 in3
		Bending, S_{act} =	3.361 in3
	Therefore....	Sact < Sreq	(OK)
	Pipe Size =	4.500 in X 0.2500 in	Rd. Aluminum Pipe, S = 3.3611 in3
	Data Taken From Machinery Handbook		

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NOT TO SCALE



Name: Don Bell Signs

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Description: Footing Design for Monument Sign

Assumptions:	1. Unit Weight, Γ_{SAND} =	105.00 lbs / ft ³
	2. Friction Angle, ϕ =	30 degrees
	3. Soil Bearing Capacity, q_s =	2200.00 lbs / ft ²
	4. Factor of Safety =	3.00

Analysis:

1. Broms Method

> Shear Force

$V_x = WF \frac{x^2}{L^2}$, where

W = Total Wind Force =	1209.07 lbs
$x = \frac{2}{3} \times L =$	8.67
L = Total Height of Sign =	13.00

Therefore... $V_x =$ 537.37 lbs / 4 Columns = 134.34 lbs

> Diameter, D =

$D = \frac{2V_x(e+L)}{\Gamma L^3 K_p}$, where

Diameter of Footing, D =	2.00 ft
Shear Wind Force, V_x =	134.34 lbs
Moment Arm, e =	6.50 ft
Requ. Embedment Depth, L_R =	3.25 ft
Exist Embed Depth, L_E =	N/A
Unit Weight of Sand, Γ =	105.00 lbs / ft ³
Passive Earth Coef., K_p =	0.5
Depth Check =	OK

Therefore... Required Dia. = 2.00 ft
Exist. Dia. = N/A

Therefore...	Footing Depth =	3.25 ft
	Footing Diameter =	2.00 ft
	Volume of Concrete =	1.66 CU YDS

X 145 lbs / ft³ = 6514.11 lbs

> Bearing Pressure

$q_{\text{actual}} = \frac{W_{\text{TOTAL}} + \Pi r^2 \times d \times \Gamma_c}{\Pi r^2}$, where

$W_{\text{TOTAL}} = W_{\text{SIGN}} + W_{\text{FOOTING}} + W_{\text{POLES}} =$	1718.56 lbs
$r =$	1.00 ft
$\Gamma_{\text{concrete}} =$	145.00 pcf

$q_{\text{actual}} =$ 275.0 lbs / ft²

$q_{\text{allowable}} = q_s [1 + 0.20(d-1)]$, where

Soil Bearing Capacity, $q_s =$ 2200.0 lbs / ft²

$q_{\text{allowable}} =$ 1063.3 lbs / ft²

Therefore... $q_{\text{allowable}} > q_{\text{actual}}$ OK

Footing Size = 2.00 ft dia. @ 3.25 ft depth

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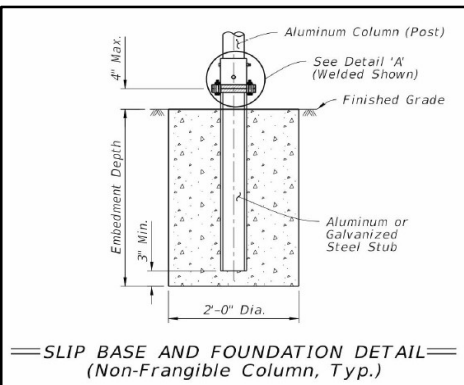
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Footing Options:



Required Footing Option:
2'-0" Diameter Footing @ 3'-3" (1.70 CU YDS Concrete)

- Notes:
- In the event utilities are encountered during footing excavation/installation, stop work immediately & notify the engineer.
 - In the event utilities are encountered, there will be two options:
 - Bottom of footing to be 3'-6" min. (horizontal distance) from top of pipe(s).
 - Concrete encase utilities according to engineer's recommendation.
 - In the event construction debris and/or other deleterious material encountered during footing excavation/installation, stop work immediately & notify the engineer.
 - Refer to 9. on S-1 for soil bearing capacity requirements. Soil must be compacted to RC = 98% min. around footing to extent 4' beyond footing diameter.
 - Concrete Volume calculations are based on neatline footings. Additional concrete may be necessary. Contractor will need to verify actual concrete volumes required.
 - In the event the footing to be hand dug and, therefore; the diameter is an approximation & not an exact shape.
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